

DRAINAGE CALCULATIONS

FOR

517 Bergen Street
LOT 28 in BLOCK 155
TOWN OF HARRISON
HUDSON COUNTY
NEW JERSEY
2022/0322

Prepared for:

Selgrove Group, LLC

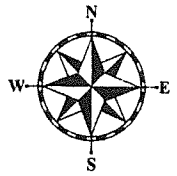
April 4, 2022

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GB ENGINEERING, LLC

Engineering • Surveying



144 Jewell Street • Garfield, NJ 07026

Tel: 973-340-0948 • Fax: 973-340-0015

Present Existing Site:

Area in Question consists of Existing Lot 28

Area = 2500 sf = 0.057 Acres

tc = 6 min.

2	year storm,	i = 5.2 "/hr;	Use Rational Method
10	year storm,	i = 6.7 "/hr;	"
100	year storm,	i = 9.1 "/hr;	"

EXISTING COEFFICIENT FROM PRESET SITE:

	Area(SF)	%	Ce	Partial Ce
Dwelling	803	32.1	0.95	0.305
Plat/Steps (Front)	52	2.1	0.95	0.020
Plat/Steps (Rear)	24	1.0	0.95	0.009
Bsmt Door	27	1.1	0.95	0.010
Conc. Feat.	345	13.8	0.95	0.131
Landscape	<u>1249</u>	<u>50.0</u>	0.3	<u>0.150</u>
	2,500	100%		0.625

RUNOFF FROM EXISTING SITE:

$$Q = C_i A = \frac{0.625}{e2} \times 5.2 \times 0.057 = 0.187 \text{ cfs} \quad 50\% = 0.093 \text{ cfs}$$

$$V = \frac{0.093}{e2} \times \frac{1}{2} \times 18.00 \times 60 = \underline{50} \quad \text{allowable volume}$$

$$Q = C_i A = \frac{0.625}{e10} \times 6.7 \times 0.057 = 0.240 \text{ cfs} \quad 75\% = 0.180 \text{ cfs}$$

$$V = \frac{0.180}{e10} \times \frac{1}{2} \times 18.00 \times 60 = \underline{97} \quad \text{allowable volume}$$

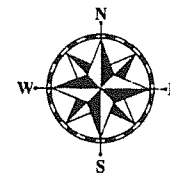
$$Q = C_i A = \frac{0.625}{e100} \times 9.1 \times 0.057 = 0.327 \text{ cfs} \quad 80\% = 0.261 \text{ cfs}$$

$$V = 0.261 \times \frac{1}{2} \times 18.00 \times 60 = \underline{141} \quad \text{allowable volume}$$



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Proposed Developed Site:

Area in Question consists of Proposed Lot 28

Area = 2500 sf = 0.057 Acres

tc = 6 min.

2	year storm,	i = 5.2 "/hr;	Use Rational Method
10	year storm,	i = 6.7 "/hr;	"
100	year storm,	i = 9.1 "/hr;	"

RUNOFF COEFFICIENT FROM PROPOSED SITE:

	Area(SF)	%	Ce	Partial Ce
Dwelling	1003	40.1	0.95	0.381
Front Plat/Steps	77	3.1	0.95	0.029
Driveway	310	12.4	0.95	0.118
Conc Features	218	8.7	0.95	0.083
AC Pad	18	0.7	0.95	0.007
Landscape	<u>874</u>	<u>35.0</u>	0.3	<u>0.105</u>
	2500	100%		0.723

RUNOFF FROM PROPOSED SITE:

$$Q = \frac{0.723}{p^2} \times 5.2 \times 0.057 = 0.216 \text{ cfs}$$

$$V = \frac{0.216}{p^2} \times \frac{1}{2} \times 18.00 \times 60 = \underline{116} \text{ cf}$$

$$Q = \frac{0.723}{p^{10}} \times 6.7 \times 0.057 = 0.278 \text{ cfs}$$

$$V = \frac{0.278}{p^{10}} \times \frac{1}{2} \times 18.00 \times 60 = \underline{150} \text{ cf}$$

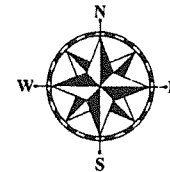
$$Q = \frac{0.723}{p^{100}} \times 9.1 \times 0.057 = 0.377 \text{ cfs}$$

$$V = \frac{0.377}{p^{100}} \times \frac{1}{2} \times 18.00 \times 60 = \underline{204} \text{ cf}$$



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Storage Required for Proposed Increase in Runoff Volume: (as per Residential Site Improvement)

2 yr storm 116 cf - 50 = 66 cf

10 yr storm 150 cf - 97 = 53 cf

100 yr storm 204 cf - 141 = 63 cf

Storage Required for Roof Runoff:

10 yr storm used 60 min. storm duration I = 2.0"/hr

Roof Area - 1003 sf.

Designated Area 1003 x $\frac{2.0}{12}$ x 0.95 x $\frac{60}{60}$ = 159 cf.

Note: 66 cf (req. for 2 yr storm) < 159 cf

Use 159 cf as design volume

SEEPAGE PIT STORAGE CALCULATIONS FOR ROOF RUNOFF

Pre-cast 7.5 ft inner diameter concrete seepage pit → 3 ft deep
(8 ft outer diameter)

Provide stone with filter fabric around pit → 2.0 ft thick

40% stone void ratio used.

* No base stone area considered as storage due to sedimentation.

Pit Volume: 3 ft deep 2.0 ft thick stone

V = (3.14 x 7.5 x 8 x 3 / 4) +

(3.14 x (144 - 64) x 0.4 x 3 / 4 = 207.9 CF

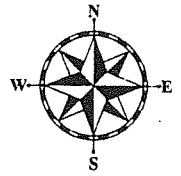
208 CF PIT

Provided Storage for Dwelling 208 CF > 159 CF Required



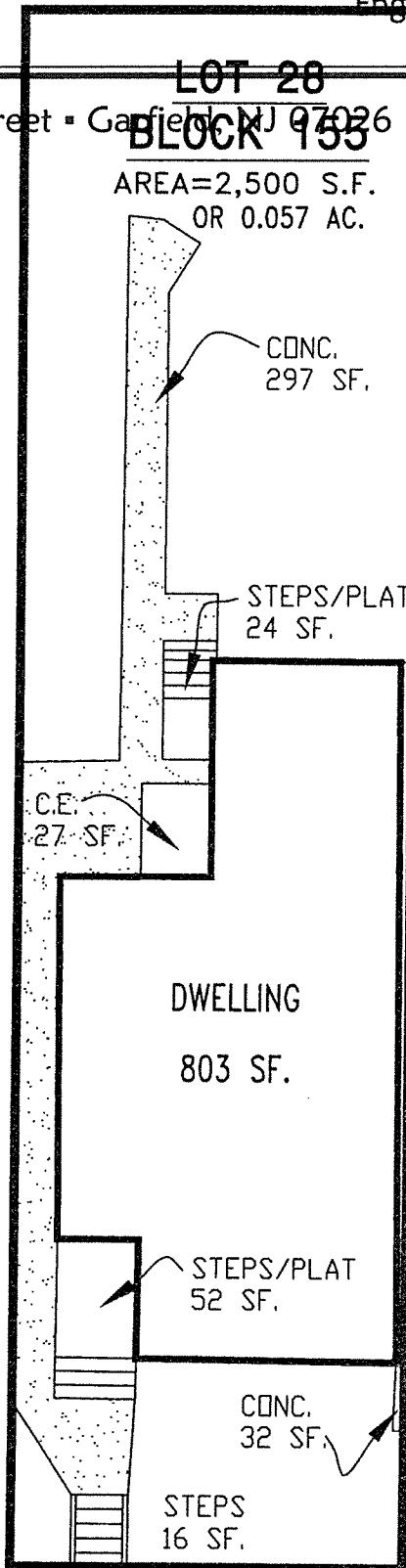
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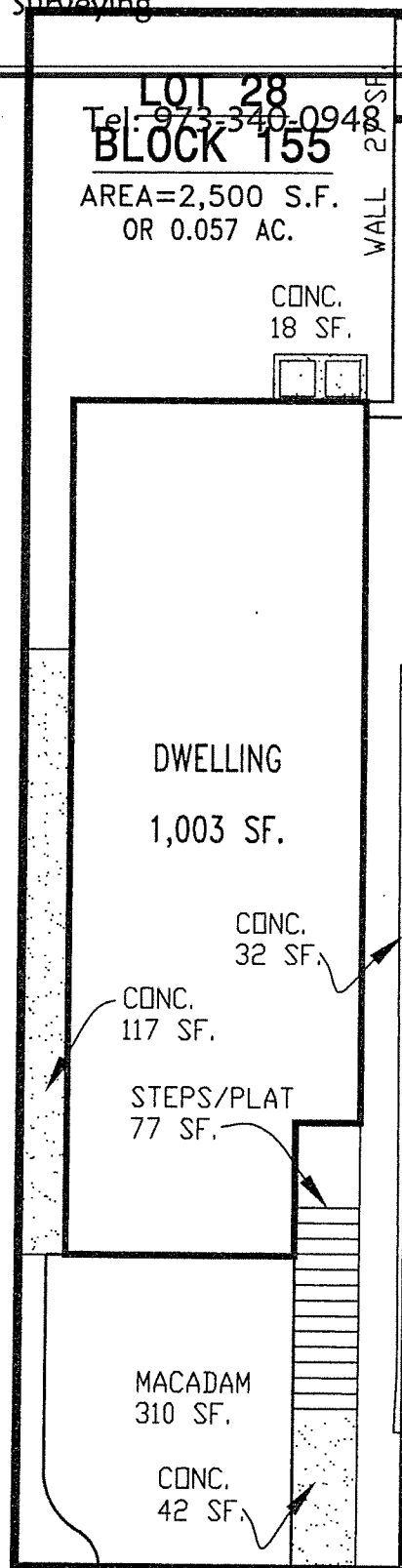


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EXISTING



PROPOSED